PROCEEDINGS OF THE 2ND NORTH AMERICAN ROCK MECHANICS SYMPOSIUM: NARMS'96 A REGIONAL CONFERENCE OF ISRM/MONTRÉAL/QUÉBEC/CANADA/19-21 JUNE 1996

Rock Mechanics Tools and Techniques

Edited by

MICHEL AUBERTIN

Ecole Polytechnique, Montréal, Québec, Canada

FERRI HASSANI

McGill University, Montréal, Québec, Canada

HANI MITRI

McGill University, Montréal, Québec, Canada

OFFPRINT



A.A.BALKEMA/ROTTERDAM/BROOKFIELD/1996

Incipient interface waves used to monitor rock failure

Laura J. Pyrak-Nolte, Sanjit Roy & Christopher Neumann University of Notre Dame, Ind., USA

ABSTRACT: We show that shear waves are especially sensitive to crack formation when the wave particle motion is perpendicular to the fracture plane, because these shear waves couple into interface waves that propagate along a fracture. Long before catastrophic failure, when a macroscopic fracture is formed, the energy in shear wave signals shows a dramatic frequency shift. This frequency shift is a signature of the partitioning of energy out of a bulk wave and into interface waves. Because this signature is observed prior to failure, it suggests the presence of an incipient interface wave that is supported by the network of oriented but disconnected micro-cracks.

1 INTRODUCTION

Seismic wave propagation through rock masses approaching failure is affected by the formation of micro-cracks, and appears as a modification in the modulus of the rock which affects seismic velocities and introduces anisotropy (Rothman, 1975; O'Connell & Budianski, 1974; Jaeger & Cook, 1979; Crampin, 1981; Zheng, 1989; Sayers, 1990; Hudson, 1991). However, more specific information about the approach to failure can be obtained by taking advantage of the tendency for microcracks to predominantly orient along the plane of principal stress. Seismic waves propagating along the plane of failure would lead to the inception of interface waves that would propagate along the plane of weakness and that would be sensitive to changes in the network of oriented but discontinuous microcracks.

The characteristics of interface waves that propagate along fractures are now well established (Pyrak-Nolte & Cook, 1987; Nagy, 1991; Pyrak-Nolte et al., 1992; Gu, 1994; Ekern et al., 1995). It has also been shown that these fractures are sensitive to fracture properties, making them sensitive probes of the physical condition of the fracture (Pyrak-Nolte et al., 1992; Roy & Pyrak-Nolte, 1995; Ekern et al., 1995). For instance, the velocity of interface waves depends on the specific stiffness of the One signature of interface wave propagation is that the frequency content of the signal is affected by the fracture stiffness, specifically, the dominant seismic energy shifts to lower frequencies as the fracture stiffness decreases. For a rock undergoing failure, where the micro-cracks are orient

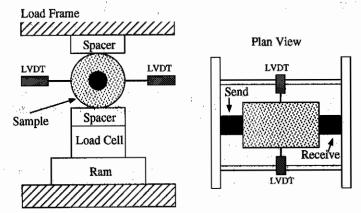


Figure 1. Sketch of experimental set-up.

along a plane, the stiffness of this plane would decrease as failure is approached, and the frequency content of an interface wave would be expected to shift to lower frequencies. In this paper, we investigate the seismic signature of the inception of interface waves during the growth of microcracks from Brazil-mode failure.

2 EXPERIMENTAL SET-UP

Several cores of Berea sandstone were used to ascertain the seismic precursor to failure. Table 1 gives the dimensions of the cores, BS15 and BS16, presented in this paper. A sketch of the experimental set-up is shown in Figure 1. The sample is placed in a load frame which applies a line load parallel to the long axis of the core, similar to Brazil testing (Jaeger & Cook, 1979) or split cylinder testing. Piezoelectric transducers for sending and receiving seismic waves are attached to the core by a small frame and are coupled to the core with honey. Two polarizations of shear wave (S_L & S_{II}) transducers, with a central frequency of 1.0 MHz, were used to monitor the failure process during Brazil-mode fracturing of the cores. Sil polarization was taken parallel to the line of loading and S₁ polarization was taken perpendicular to the line of loading. S_L was measured on sample BS15 and S_{II} was measured on BS16. Linear displacement transducers (LVDTs) with sub-micron resolution were used to measured the diametrical expansion of the core during loading and were mounted diametrically opposed using the same frame that held the piezoelectric transducers (Figure 1), A load cell was used to measure the load on the sample. The waveform, displacement, and load were collected concurrently with a four-channel digital oscilloscope to allow exact temporal correlation.

Table 1. Sample dimensions		<u> </u>
Sample	Length (cm)	Diameter (cm)
BS15	4.52	5.05
BS16	4.31	5.05

3 RESULTS

Figures 2 & 3 show the received S_{\perp} and S_{\parallel} -waveforms as a function of load for sample BS15 and BS16, respectively. As failure is approach, the S_{\perp} -wave is delayed, attenuated, and the phase of the signal is altered. The arrows in Figure 2 indicate the emergence of an interface wave starting at an applied load of 17.9 kN. At this load, a change in slope is observed corresponding to the inception of an interface wave. The displacement as a function of

load for sample BS15 is shown in Figure 6. The change in slope around 14 kN represents the increase in displacement caused by crack growth, with failure occurring at 19 kN. The observation of the emergence of an interface wave at load of 17.9 kN suggests that enough cracks have formed to weaken the plane of failure. For the S_{II}-wave which is known not to couple into interface waves, the velocity increased but no attenuation and no phase changes of the wave is observed until post-failure (Figure 3). Sample BS16 therefore performs as a control experiment. From theoretical analysis and experimental measurements (Pyrak-Nolte & Cook, 1987; Pyrak-Nolte et al., 1992; Gu, 1994), it has been shown that S_{II} excitation on a fracture will not generate interface waves. We can therefore use S_{II} waves as a control experiment to show that the signatures that we see for S1 are absent.

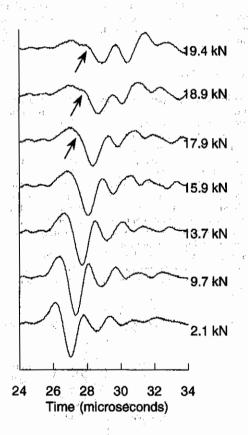


Figure 2. Received S_{\perp} waveforms for a range of loads. Arrows indicate the inception of an interface wave.

The frequency content of an interface wave depends on the specific stiffness of the fracture. Roy & Pyrak-Nolte (1995) have shown that on pre-existing fractures, as fracture specific stiffness increases, the dominant energy in an interface wave shifts to higher frequencies. During the Brazil-mode failure process, a plane of weakness is induced as the load on the sample is increased. This plane of weakness consists of a distribution of microcracks that reduce the stiffness of this plane. If the observed waves are interface waves, a reduction in the frequency content of the signal would be expected. A wavelet analysis

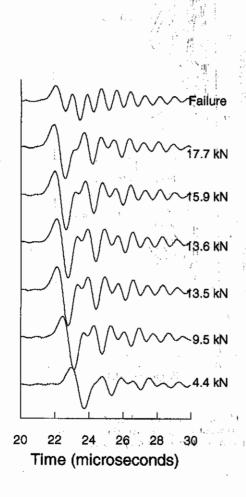


Figure 3. Received S_{II} waveforms for a range of loads.

(Combes et al., 1989; Pyrak-Nolte & Nolte, 1995) of the received waveforms was performed to examine the energy partitioning during failure. The wavelet analysis provides a direct quantitative measure of spectral content as a function of arrival time, and is used to look for the signature of an interface wave, namely the spectral shift with change in stiffness.

Figures 4 & 5 show the group wavelet transformation for samples BS15 and the control BS16 for several loads. The contour represents lines of constant amplitude. For S1 -waveforms from sample BS15, as the load on the sample increased from 13.7 kN to 17.9 kN, there is a large shift of the dominant energy from 0.58 MHz to 0.21 MHz. This shift in frequency content is consistent with the generation and existence of interface waves (Nagy, 1991; Pyrak-Nolte et al., 1992; Gu, 1994; Pyrak-Nolte and Nolte, 1995; Roy and Pyrak-Nolte. 1995), and is a signature of the partitioning of energy out of a bulk wave and into interface waves. Because this signature is observed prior to failure, it suggests the presence of an incipient interface wave that is supported by the network of oriented but discontinuous micro-cracks that form during loading.

For S_{II} -waveforms from sample BS16, the dominant energy exhibits a small shift to higher frequencies prior to failure (Figure 5). The spectral peak was observed to shift from 0.58 MHz at a load of 2.7 kN to 0.62 MHz for loads greater than 4.0 kN, and after failure the dominant energy occurred at 1.1 MHz. This increasing frequency behavior is

opposite that expected for interface waves.

For samples BS15 and BS16, the energy partitioning between low and high frequency components of the signal is examined for two frequencies. Figure 6 shows the change in amplitude as a function of load for frequencies of 0.21 MHz and 0.54 MHz for sample BS15. The displacement as a function of load is also shown in Figure 6 for comparison. Initially, both the low and the high frequency energy increase with increasing load. A decrease in high frequency energy begins approximately at the same load when cracks begin to grow around 14 kN, and decreases by 65% just prior to failure. However, the low frequency energy component gains strength with increasing load and increases in amplitude by 120% between the initiation of crack growth and failure. As cracks grow along the plane of failure, the stiffness of this plane decreases which leads to the inception of interface waves. The growth of cracks is also indicated by a change in group velocity of the energy traveling at these two frequencies. As the sample deforms, the low and high frequency components of the energy are both delayed.

Examination of the spectral content for the S_{II}waveforms show a spectral shift to higher frequencies as the sample is loaded to failure (Figure 5). For a frequency of 0.66 MHz and 1.18 MHz, the

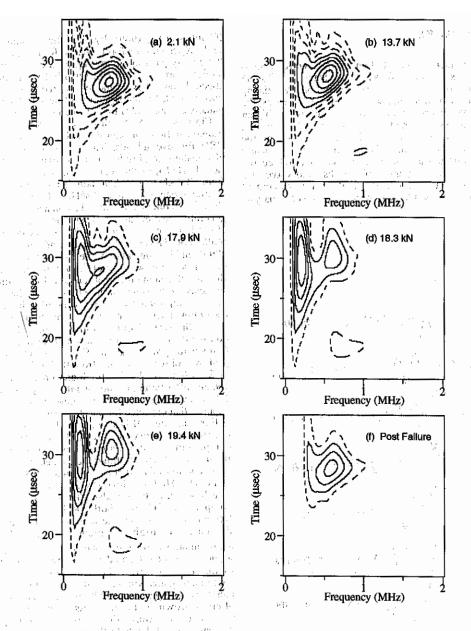


Figure 4. Group wavelet transformation of received S_⊥ waveforms for selected stresses. Contour ranges and intervals are (a)-(b) 10-80 in intervals of 10; (c)-(e) 10-50 in intervals of 10; (f) 10-40 in intervals of 10. Units of contours are Volts per root Megahertz.

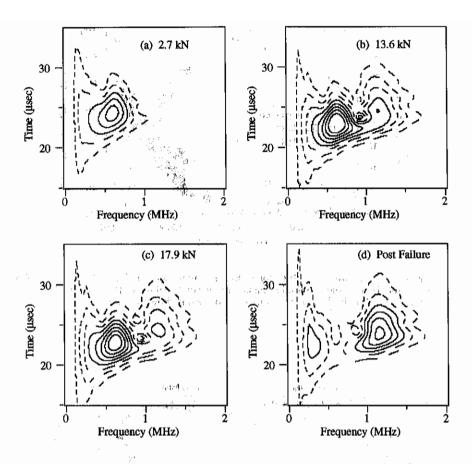


Figure 5. Group wavelet transformation of received S_{II} waveforms for selected stresses. Contour ranges and intervals are (a) 25-125 in intervals of 25; (b)-(c) 25-200 in intervals of 25; (e) 25-150 in intervals of 25. Units of contours are Volts per root Megahertz.

energy at both these frequencies decreased only 10%-15% between a load of 15 kN (when crack growth begins) and failure. As observed in the received waveforms (Figure 4), the velocity of the S_{||}-wave increased with increasing load until crack initiation occurred. After crack initiation, the velocity at both frequency components plateaued. For this shear wave polarization, i.e. parallel to loading, the shear wave is sampling grain contacts that increase in stiffness with increasing load. From theoretical modeling of a single non-welded contact (Pyrak-Nolte et al., 1990; Nihei, 1992), an increase in stiffness of a non-welded contact increases the amplitude and velocity of the transmitted wave, and shifts the spectral content to higher frequencies.

5 SUMMARY

The seismic signature of the initial formation of a failure plane is the generation of incipient interface waves. These waves show a distinct shift in wave energy to lower frequencies as failure is approached. Furthermore, because the incipient interface wave is strongly sensitive to fracture properties, this provides a sensitive technique for monitoring the growth of microcracks during rock failure.

Acknowledgments: This work was supported by the Department of Energy (DE-F602-93 ER14391). LJPN would also like to acknowledge the Young Investigator award of the NSF. The authors would also like to acknowledge the useful discussions with D. D. Nolte on various aspects of this projects.

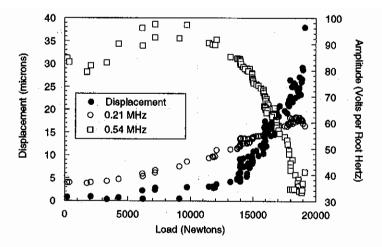


Figure 6. S_{\perp} amplitude as a function of load for frequencies of 0.21 MHz and 0.54 MHz for sample BS15. The amplitude data is form wavelet analysis of the received S_{\perp} waveforms. The displacement as a function of load is shown for comparison.

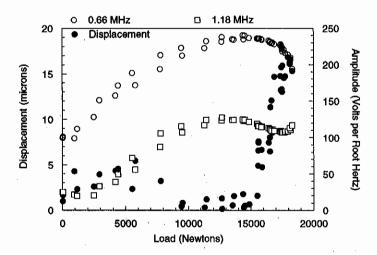


Figure 7. S_{\parallel} amplitude as a function of load for frequencies of 0.66 MHz and 1.18 MHz for sample BS16. The amplitude data is form wavelet analysis of the received S_{\parallel} waveforms. The displacement as a function of load is shown for comparison.

REFERENCES

Combes, J. M., A. Grossman and Ph. Tchamitchian, eds. 1989. Wavelets: Time-Frequency Methods and Phase Space. Berlin: Springer-Verlag.

Crampin, S. R. 1981. A review of wave motion in anisotropic and cracked elastic media. Wave

Motion. 3:343-391.

Ekern A., Suarez-Rivera R., and A. Hansen. 1995. Investigation of interface wave propagation along planar fractures in sedimentary rocks. Proceedings of the 35th US Symposium on Rock Mechanics. Rotterdam: Balkema.

Gu, B. 1994. Interface Waves on a Fracture, Ph.D. Thesis. University of California, Berkelev.

Hudson, J. A. 1991. Crack distributions which account for a given seismic anisotropy. Geophysical Journal International. 104:517-521.

Jaeger, J. C. and N. G. W. Cook. 1979. Fundamentals of Rock Mechanics. 3rd Edition.

London: Chapman and Hall. Nagy, P. B. 1991. Ultrasonic detection of kissing bonds at adhesive interfaces. Journal of Adhesion Science and Technology. 5:619-630.

Nihei, K. T. 1992. Micromechanics of Seismic Wave Propagation in Granular Rocks. Ph.D. Thesis. University of California, Berkeley.

O'Connell, R. J. and B. Budianski. 1974. Seismic velocities in dry and saturated cracked solids. Journal of Geophysical Research. 79:5412.

Pyrak-Nolte, L.J. and N.G.W. Cook. 1987. Elastic Interface Waves along a Fracture. Geophysical

Research Letters. 14:1107-1110.

Pyrak-Nolte, L. J., Myer, L.R. and N.G.W. Cook. 1990. Transmission of seismic waves across single natural fractures. Journal Geophysical Research. 95:8617-8638.

Pyrak-Nolte, L. J. and D. D. Nolte. 1995. Wavelet analysis of velocity dispersion of interface waves along fractures, Geophysical Research Letters. 22:1329-1332.

Rothman, R. L. 1975. Crack Distributions under Uniaxial Load and Associated Changes in Seismic Velocities Prior to Failure, Ph.D. Thesis, The Pennsylvania State University. 131p.

Roy, S. and L. J. Pyrak-Nolte. 1995. Interface waves along tensile fractures in dolomite, Geophysical Research Letters. 22:2773-2776.

Sayers, C. M. 1990. Inversion of ultrasonic wave velocity measurements to obtain the microcrack orientation distribution function in rocks. Engineering Fracture Mechanics. 35:743-749.

Zheng, Z. 1989. Compressive Stess-Induced Microcracks in Rocks and Applications to Seismic Anisotropy. Ph.D. Thesis. University of California, Berkeley.

FROM THE SAME PUBLISHER:

Fuji, T. (ed.)

90 5410 573 9
8th international congress on rock mechanics — Proceedings /
Comptes-rendus / Berichte Tokyo, Japan, 25-30 September 1995
1995-96, 30 cm, c.2000 pp. 3 vols., Hfl.650 / \$420 / £265
The official proceedings of the 8th congress of the International
Society for Rock Mechanics, held every 4 years. Topics: Geology,
site exploration and testing; Physical properties and modelling of
rock; Near surface excavations; Stability of slopes and foundations; Excavation and the stability of underground openings; Hest,
water flow, and chemical transport in rock masses; Information
systems and new technologies relating to rock mechanics; etc.

9054103808 Nelson, P.P. & S.E.Laubach (eds.) Rock mechanics: Models and measurements, Challenges from industry - Proceedings of the 1st North American rock mechanics symposium, Austin, Texas, 1-3 June 1994 1994, 22 cm, 1200 pp., Hfl.150/\$85.00/£55 Keynote lectures; Panel discussion; Hydraulic & mechanical properties of discontinuities; Borehole stability & hydraulic fracture: Rock engineering for underground excavations; New developments in blasting; Rock cutting & TBM performance simulation; In-situ stress; Mechanical properties of intact rock; Static & dynamic properties of intact rock; Uncertainty & reliability in rock engineering; Numerical modelling; Mechanics of weak rock; Intact rock testing; Natural fracture systems; Mechanics of poorly consolidated weak rock; Deep mine design in South Africa & Canada; Deep mine design/bursting/pillar design; Coal & surface mine design/Rock mechanics in dam & hydro-project construction; Rock mechanics in the design of rock slopes; Index, Editors: Univ. of Texas at Austin, USA.

Daemen, Jaak J.K. & Richard A.Schultz (eds.) 90 5410 552 6 Rock mechanics: Proceedings of the 35th US Symposium -Lake Tahoe, 4-7 June 1995

1995, 25 cm, 950 pp., Hfl.175/\$110.00/£70

This volume contains a total of 134 papers selected from 241 abstracts, in addition to 2 invited papers prepared on broad multidisciplinary issues. Session topics include: Construction; surface excavation, stability and shear strength of fractured rock; laboratory testing; rock dynamics; stress measurements; tunnels and groundwater flow; petroleum; tool-rock interaction; building stone durability; rock reinforcement; fracture mechanics; radioactive waste disposal; underground mining; fragmentation and blasting; metal mining; coal mining; planetary rock mechanics; rock properties; stochastic methods; theoretical and model studies; hydrology; and rock creep; etc.

Yoshinaka, R. & K.Kikuchi (eds.)

90 5410 562 3

Rock foundations — Proceedings of the international workshop on Rock foundations, Tokyo, Japan, 30 September 1995 1995, 30 cm, 468 pp., Hfl. 190/\$120.00 /£76

The function of rock foundations is to support large scale and/or heavy structures such as high dam, long-span bridge, nuclear power station, highrise building or pylon, and without risk to carry the heavy load of superstructures. Those structures founded on rock, generally constitute the important infrastructures for the highly developed urban life. Due to the presence of discontinuities and weak layers, rock masses are intrinsically discontinuous and/or inhomogeneous. Therefore, the evaluation of bearing capacity, deformation behaviour of foundation of rock, stress-strain distribution, and seepage flow in rock mass below foundation, etc. is very important and intricate. This book contains the recent and comprehensive results from research work on rock foundation en-

gineering. The main topics covered are investiga-tion, testing, mechanical properties of rock and rock mass, numerical approach, bearing capacity of rock foundation, seepage flow, design methodology, improvement and reinforcement of rock mass, monitoring and behaviour of rock foundation.

Ribeiro e Sousa, L. & N.F.Grossmann (eds.) 9054103396 Safety and environmental issues in rock engineering/Problèmes de securité et d'environnement en mécanique des roches -Proceedings / Comptes-rendus / Sitzungsberichte / ISRM international symposium, EUROCK '93, Lisbon, 21-24 June 1993 1993-94, 25 cm, 900 pp., 2 vols, Hfl. 265/\$150.00/£98 State-of-the-art in science & engineering aspects of rock mechanics in the areas of safety & environmental protection. Discusses various aspects concerning modelling in safety analysis, stability of underground structures, and the contribution of incident and accident cases to the progress of rock engineering activities. The influence of the environment is also considered, namely in heat & mass transport, contaminant migration, and underground storage of waste & products. Editors: Lab. Nacional de Engenharia Civil, Lisbon.

Cripps, J.C., J.M.Coulthard, M.G.Culshaw, A.Forster, S.R.Hencher & C.F.Moon (eds.) 90 6191 1672

The engineering geology of weak rock — Proceedings of the 26th annual conference of the Engineering Group of the Geological Society, Leeds, UK, 9-13.09.1990

1993, 25 cm, 520 pp., Hfl.195/\$115.00/£72

52 refereed & edited papers: Case histories, theoretical concepts & laboratory studies; Rock behaviour; Classification of rock masses & the engineering behaviour of very complex rock masses.

Xie, Heping (M.A.Kwasniewski, Editor-in-Chief) 90 5410 133 4 Fractals in rock mechanics

(Geomechanics research series 1)

1993, 25 cm, 464 pp., Hfl.150/\$85.00/£55

Important developments in the progress of the theory of rock mechanics during recent years are based on fractals and damage mechanics. The book is concerned with these developments, as related to fractal descriptions of fragmentations, damage, and fracture in rocks, rock bursts, joint roughness, rock porosity and permeability, rock grain growth, rock and soil particles, shear slips, fluid flow through jointed rocks, faults, earthquake clustering, etc. A simple account of the basic concepts, methods of fractal geometry & their applications to rock mechanics, geology & seismology. Discussion of damage mechanics of rocks & its application to mining engineering. Author: China Univ. of Mining & Technology, Xuzhou, China

Rock mechanics in petroleum engineering – Proceedings / Comptes-rendus / Sitzungsberichtel SPE/ISRM International Conference/EUROCK 94/Delf/129-31 August 1994
1994, 25 cm, 992 pp., Hfl.180/\$99.00/£68
Besides covering established research topics like drilling, sand failure & hydraulic fracturing, the conference has focused on the more recent, environmental-related topics of waste disposal, hydrocarbon storage, subsidence & compaction induced seismicity. Also highlighted are emerging technologies for improved rock characterisation, such as downhole probes, acoustic emission & acoustic tomography. Topics: Rock characterisation and behaviour; Stability of well bores and excavations; Fracture mechanics; Rock mass response of hydrocarbon production and mining; Storage, waste disposal and environmental applications; Chalk; Insitu measurement. 103 papers.